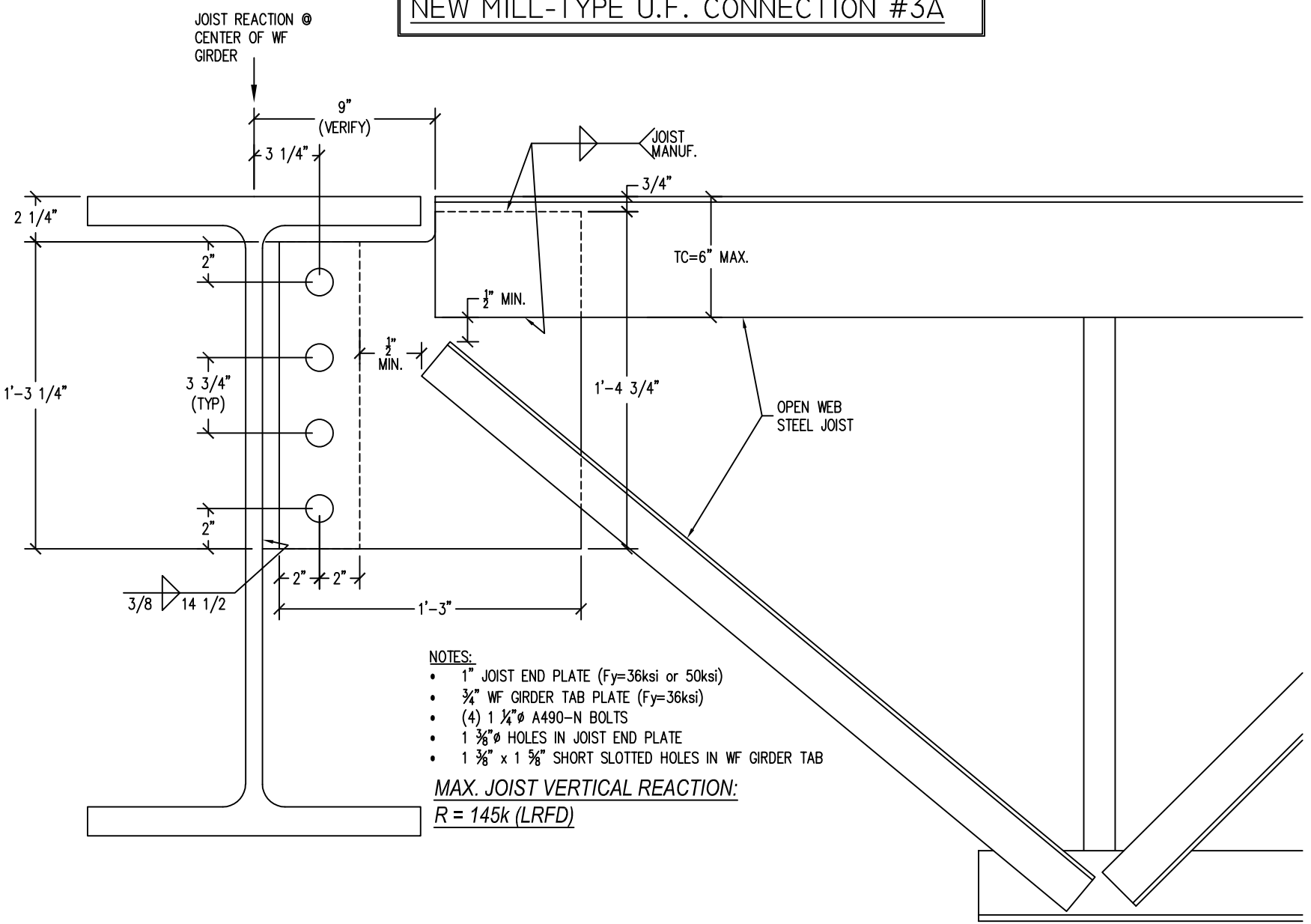
UNDER-FLANGE (UF) CONNECTION

NEW MILL-TYPE U.F. CONNECTION #3A



NOTES:

- 1" JOIST END PLATE (Fy=36ksi or 50ksi)
- ¾" WF GIRDER TAB PLATE (Fy=36ksi)
- (4) 1 ¼"ø A490-N BOLTS
- 1 ⅜"ø HOLES IN JOIST END PLATE
- 1 ⅜" x 1 ⅝" SHORT SLOTTED HOLES IN WF GIRDER TAB

MAX. JOIST VERTICAL REACTION:
R = 145k (LRFD)

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Date: 3/18/2025

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JOB NAME:	JOB #:		
LOCATION:	NMBS U.F. #3A Joist End Plate	DATE:	3/18/2025

AISC 16TH - p. 10-49 (Extended Configuration)

Holes must satisfy AISC J3.3

Horizontal Axial forces (seismic or wind) if present, to be transferred from beam to joist via tie plate

Joist Plate, Fu =	58	ksi	TC Hold Back Distance (H) =	9	in	Joist Plate w =	15 in
Joist Tab Plate, Fy =	36	ksi	TC Angle Size =	3.5	in	Joist Plate d =	15.25 in
Joist Plate, d or w=	15	in	Joist Plate Hold-Down from TC =	0.75	in		
Joist Plate Thickness, t=	1	in					
Joist Plate Edge Distances, de=	2	in					
e =	3.25	in				L_{i1} =	5.625
Vertical Shear , Vu=	145	k (LRFD)				L_{i2} =	1.875
Vertical Ecc. Moment , Mu=	471.25	k*in (LRFD)				L_{i3} =	0
Bolt Diameter, Db =	1.25	in				L_{i4} =	0
Bolt Shear Capacity ϕRn =	62.7	k (A490-N)				L_{i5} =	0
# of Bolts, Nb=	4	(Spreadsheet design limitation, max. 10 bolts)					
Spacing of Bolt Group, S=	3.75	in					
Vert. C.G. of Bolt Group =	5.625	in					
Fnv/0.9 =	75.56	ksi (Table J3.2, A490-N Bolts)					
Short Slotted Hole, Lh =	1.63	in (conservative design, allows for slotted holes in joist plate)					
Joist Top Chord Axial Force, V_{TC}=	217.5	k (LRFD)...**Assumes 1.5:1 End Web Slope**					

Gross Plate Area, Ag =	15	in^2
Effective Plate Area, Ae =	13.63	in^2
Z =	56.25	in^3 (1/4t*w^2)
S =	37.5	in^3 (1/6t*w^2)

Bolt Shear - Elastic Vector Method: (AISC p. 7-7, 7-8)

Bolt Group I_p =	70.31	in4/in2
r_{py} =	36.25	k (Vu/# Bolts)
r_{mx} =	37.70	k ($Mu*L_{i1}/I_p$)
Hm =	37.70	k (rmx *Nc) Nc = 1 column of bolts
Ru =	52.30	k $(r_{py}^2+r_{mx}^2)^{1/2}$
Ru / ϕRn =	**0.83**	**< 1.0 OK**

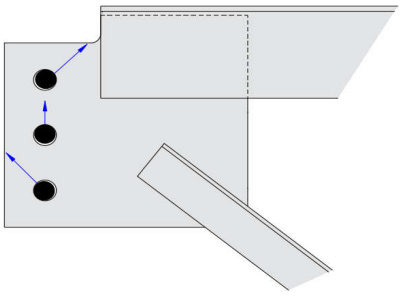
Bolt Bearing & Tearout: (AISC J3.11)

Bearing Rnb =	174.00	k/bolt (2.4*Db*t*Fu) AISC J3-6a
L_{ch} =	1.19	in (d_e - Lh/2)
Horizontal Tearout Rnt =	82.65	k/bolt ($1.2*L_{ch}$*t*Fu) AISC J3-6c
Ru =	52.30	k (worst case bolt shear)
r_{mx} =	37.70	k (worst case horiz. bolt shear)
ϕ =	0.75	(AISC J3.11)
Bearing Ru / ϕRnb =	**0.40**	**< 1.0 OK**
Tearout r_{mx} / ϕRnt =	**0.61**	**< 1.0 OK**

Stress Ratio Results:	
Bolt Shear (V&M):	**0.83**
Bolt Bearing & Tearout:	**0.61**
Shear Plate Rupture:	**0.22**
Shear Plate Block Shear:	**0.61**
Shear Plate Flexural Rupture:	**0.26**
Shear Plate Yielding & Flexural:	**0.27**

Min. Joist TC to Plate Weld:

6 /16th x 7 " Fillet Weld



NOTE: FOR VISUAL FORCE SCHEMATIC ONLY

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JOB NAME:	JOB #:		
LOCATION:	NMBS U.F. #3A Joist End Plate	DATE:	3/18/2025

Shear Plate Rupture: (AISC J4.2)

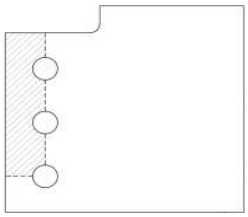
Crushed Hole Width, W' = 1.4375 in (plate hole + 1/16" Crushed width)
Net Plastic Modulus, Z_{net} = 46.50 in^3 (Z - W'*t*d_{hole}) d_{hole} = 6.78125
ϕVn = 355.61 k (ϕ = 0.75, ϕ*.60*Fu*Ae) AISC J4-4
ϕMn = 2022.83 k*in (ϕ = 0.75, ϕ*Fu*Z)
Ru / ϕRn = **0.22** **< 1.0 OK** $(Vu/\phi Vn)^2+(Mu/\phi Mn)^2$

Shear Plate Block Shear: (AISC J4.3)

Vertical Direction

Gross Area in Shear, Agv= 13.25 in^2 (t*(d_e+(Nb-1)*S)
Net Area in Shear, Anv= 7.50 in^2 Agv-(Nb*W')*t
Net Area in Tension, Ant= 1.19 in^2 (t*(d_e-Lh/2)
Gross Area, ϕRn = 266.31 k (0.75*(0.6*Fy*Agv+Ubs*Fu*Ant)) Ubs = 1.0 for single bolt line
Net Area, ϕRn = 247.41 k (0.75*(0.6*Fu*Anv+Ubs*Fu*Ant)) AISC J4-5

Note: Use of Lh for determination of Net Plate Area, allows for the slots to be in either the joist end plate or the beam tab.

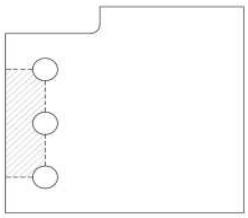


NOTE: SCHEMATIC ONLY, HOLES OR PLATE MAY VARY

Horizontal Direction

Gross Area in Shear, Agv= 4.00 in^2 (2*t*d_e)
Net Area in Shear, Anv= 2.38 in^2 (2*t*(d_e-Lh/2)
Net Area in Tension, Ant= 6.94 in^2 (t*((Nb-1)*S-(Nb-1)*W)
Gross Area, ϕRn = 366.58 k (0.75*(0.6*Fy*Agv+Ubs*Fu*Ant)) Ubs = 1.0 for single bolt line
Net Area, ϕRn = 363.77 k (0.75*(0.6*Fu*Anv+Ubs*Fu*Ant)) AISC J4-5

Note: Use of Lh for determination of Net Plate Area, allows for the slots to be in either the joist end plate or the beam tab.



ϕRn = **247.41 k** **Controls**
Ru / ϕRn = 0.61 < 1.0 OK $(Vu^2+Hm^2)^{1/2}/\phi Rn$

Shear Plate Flexural Rupture: (AISC F11)

Yielding Mc = 1822.5 k*in (ϕ = 0.9, ϕ*(Fy*Z<1.5Fy*S)) AISC F11-1
Lateral-Torsional Buckling Check: 48.8 Lb*d/t^2, Lb = e <0.08*E/Fy Lateral Torsional Buckling does not apply
Lateral-Torsional Buckling, Mc = N/A k*in
Ru / ϕRn = **0.26** **< 1.0 OK** (Mu/Mc)

Shear Plate Yielding & Flexural Strength: (AISC 10-8)

Vc = 324 k (ϕ = 1.0, ϕ*0.6*Fy*Ag) AISC J4-3
Yielding Mc = 1822.5 k*in (ϕ = 0.9, ϕ*(Fy*Z<1.5Fy*S)) AISC F11-1
Lateral-Torsional Buckling Check: 48.8 Lb*d/t^2, Lb = e <0.08*E/Fy Lateral Torsional Buckling does not apply
Lateral-Torsional Buckling, Mc = N/A k*in
Ru / ϕRn = **0.27** **< 1.0 OK** $(Vu/Vc)^2+(Mu/Mc)^2$ AISC 10-8

Joist Plate Weld (Angle = 0 deg. & C_1 = 1.00 E70 Electrode):

Length of Plate Weld L_w = 7 in (w-(H-1.25)-0.25")
a_y = 0.2 AISC Table 8-4 a_y = (Weld Centroid - TC Centroid) / L_w
k_y = 0.4 AISC Table 8-4 k_y = Weld Spacing / L_w
Cy = 3.47 (y-axis weld eccentricity, AISC Table 8-4)
D_{min} = **6** /16ths of an inch Fillet Weld Size (min)

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JOB NAME:		JOB #:	
LOCATION:	NMBS Beam Tab U.F. #3, U.F. #3A	DATE:	3/18/2025

AISC 16TH - p. 10-49 (Extended Configuration)

Holes must satisfy AISC J3.3

Horizontal Axial forces (seismic or wind) if present, to be transferred from beam to joist via tie plate

Beam Tab Plate, Fu =	58	ksi		
Beam Tab Plate, Fy =	36	ksi		
Beam Tab Plate Depth, d=	15.25	in		
Beam Tab Thickness, t=	0.75	in		
Beam Tab Edge Distances, d_e=	2	in	L_{i1} =	5.625
e =	3.25	in	L_{i2} =	1.875
Vertical Shear , Vu=	145	k (LRFD)	L_{i3} =	0
Vertical Ecc. Moment , Mu=	471.25	k*in (LRFD)	L_{i4} =	0
Bolt Diameter, Db =	1.25	in	L_{i5} =	0
Bolt Shear Capacity ϕRn =	62.7	k		
# of Bolts, Nb=	4	(Spreadsheet design limitation, max. 10 bolts)		
Spacing of Bolt Group, S=	3.75	in		
C.G. of Bolt Group =	5.625	in		
Fnv/0.9 =	75.56	ksi (Table J3.2, A490-N Bolts)		
Short Slotted Hole, Lh =	1.63	in		

Ab=	1.23	in^2 (Bolt Area)
C'=	14.07	AISC Eq. 7-17
Mmax =	1304.62	k*in (Fnv/0.9*Ab*C', Eq. 10-7)
Max. Beam Tab Thickness, tmax =	**0.93**	**in** (6*Mmax)/(Fy*d^2) **AISC Eq. 10-6**

Gross Plate Area, Ag =	11.4375	in^2
Effective Plate Area, Ae =	7.31	in^2
Z =	43.6055	in^3 (1/4t*d^2)
S_{net} =	29.07	in^3 (1/6t*d^2)

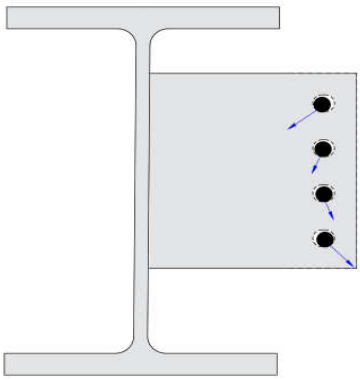
Stress Ratio Results:	
Bolt Shear (V&M):	**0.83**
Bolt Bearing & Tearout:	**0.81**
Shear Tab Rupture:	**0.73**
Shear Tab Block Shear:	**0.81**
Shear Tab Flexural Rupture:	**0.33**
Shear Tab Yielding & Flexural:	**0.46**
6 /16" Tab Weld:	**0.57**

Bolt Shear - Elastic Vector Method: (AISC p. 7-7, 7-8)

Bolt Group I_p =	70.31	in4/in2
r_{py} =	36.25	k (Vu/# Bolts)
r_{mx} =	37.70	k (Mu*L_{i1}/I_p)
Hm =	37.70	k (rmx *Nc) Nc = 1 column of bolts
Ru =	52.30	k (r_{py}^2+r_{mx}^2)$^{1/2}$
Ru / ϕRn =	**0.83**	**< 1.0 OK**

Bolt Bearing & Tearout: (AISC J3.11)

Bearing Rnb =	130.50	k/bolt (2.4*Db*t*Fu) AISC J3-6a
L_{ch} =	1.19	in (d_e - Lh/2)
Horizontal Tearout Rnt =	61.99	k/bolt (1.2*L_{ch}*t*Fu) AISC J3-6c
Ru =	52.30	k (worst case bolt shear)
r_{mx} =	37.70	k (worst case horiz. bolt shear)
ϕ =	0.75	(AISC J3.11)
Bearing Ru / ϕRnb =	**0.53**	**< 1.0 OK**
Tearout r_{mx} / ϕRnt =	**0.81**	**< 1.0 OK**



NOTE: FOR VISUAL FORCE SCHEMATIC ONLY

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JOB NAME:		JOB #:	
LOCATION:	NMBS Beam Tab U.F. #3, U.F. #3A	DATE:	3/18/2025

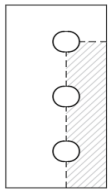
Shear Tab Rupture: (AISC J4.2)

Crushed Hole Width, W' = 1.4375 in (plate hole + 1/16" Crushed width)
Net Plastic Modulus, Z_{net} = 27.43 in^3 (Summation of A*d of net plate section)
ϕVn = 190.86 k (ϕ = 0.75, ϕ*.60*Fu*Ae) AISC J4-4
ϕMn = 1193.36 k*in (ϕ = 0.75, ϕ*Fu*Z)
Ru / ϕRn = **0.73** **< 1.0 OK** $(Vu/\phi Vn)^2+(Mu/\phi Mn)^2$

Shear Tab Block Shear: (AISC J4.3)

Vertical Direction

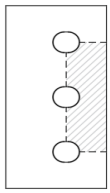
Gross Area in Shear, Agv= 9.94 in^2 ($t*(d_e+(Nb-1)*S)$
Net Area in Shear, Anv= 5.63 in^2 Agv-(Nb*W')*t
Net Area in Tension, Ant= 0.89 in^2 ($t*(d_e-Lh/2)$
Gross Area, ϕRn = 199.73 k (0.75*(0.6*Fy*Agv+Ubs*Fu*Ant)) Ubs = 1.0 for single bolt line
Net Area, ϕRn = 185.55 k (0.75*(0.6*Fu*Anv+Ubs*Fu*Ant)) AISC J4-5



NOTE: SCHEMATIC ONLY, HOLES MAY VARY

Horizontal Direction

Gross Area in Shear, Agv= 3.00 in^2 ($2*t*d_e$)
Net Area in Shear, Anv= 1.78 in^2 ($2*t*(d_e$-*Lh/2)
Net Area in Tension, Ant= 5.20 in^2 (t*((Nb-1)*S-(Nb-1)*W)
Gross Area, ϕRn = 274.94 k (0.75*(0.6*Fy*Agv+Ubs*Fu*Ant)) Ubs = 1.0 for single bolt line
Net Area, ϕRn = 272.83 k (0.75*(0.6*Fu*Anv+Ubs*Fu*Ant)) AISC J4-5



ϕRn = **185.55 k** **Controls**
Ru / ϕRn = **0.81** **< 1.0 OK** $(Vu^2+Hm^2)^{1/2}/\phi Rn$

Shear Tab Flexural Rupture: (AISC F11)

Yielding Mc = 1412.8 k*in (ϕ = 0.9, ϕ*(Fy*Z<1.5Fy*S)) AISC F11-1
Lateral-Torsional Buckling Check: 88.1 Lb*d/t^2, Lb = e Check for Lateral Torsional Buckling per AISC F11-3
Lateral-Torsional Buckling, Mc = 1412.8 k*in AISC F11-3
Ru / ϕRn = **0.33** **< 1.0 OK** (Mu/Mc)

Shear Tab Yielding & Flexural Strength: (AISC 10-8)

Vc = 247.1 k (ϕ = 1.0, ϕ*0.6*Fy*Ag) AISC J4-3
Yielding Mc = 1412.8 k*in (ϕ = 0.9, ϕ*(Fy*Z<1.5Fy*S)) AISC F11-1
Lateral-Torsional Buckling Check: 88.1 Lb*d/t^2, Lb = e Check for Lateral Torsional Buckling per AISC F11-3
Lateral-Torsional Buckling, Mc = 1412.8 k*in AISC F11-3
Ru / ϕRn = **0.46** **< 1.0 OK** $(Vu/Vc)^2+(Mu/Mc)^2$ AISC 10-8

Shear Tab Weld:

Min. Weld Thickness t_{wmin} = 0.33 in. $t_{wmin} = (t*Fy*3^{1/2})/(2*F_{EXX})$, F_{EXX} = 70ksi Electrode, AISC Engineering Journal, Vol. 46, 2009
Weld Provided t_w = 0.375 in
Min. Plate Thickness = 0.64 in (AISC Eq. 9-7, 6.19*D/Fu) **GOOD**
ϕRw = 254.72 k (ϕ*0.6*F_{EXX}*0.707*t_w*d*2)

Ru / ϕRn = **0.57** **< 1.0 OK**